

Selecting Best Management Practices using BMPTRAINS

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www.stormwater.ucf.edu







Purpose of Presentation

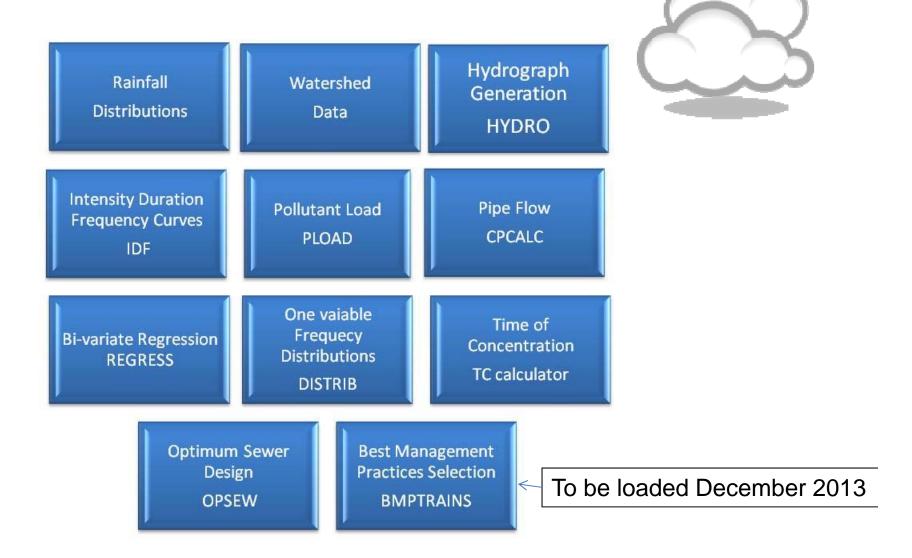
- Review methods for analysis and design of BMPs.
- Provide the basic principles for average annual removal calculations.
- Discuss a computer program for the quantification of nutrient removal that can be used as a basis for review for permits and mass reduction calculations.

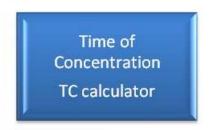




Credit and thanks to: Mike Hardin, Ikiensinma Gogo-Abite and Chris Kuzlo

SMADAonline.com (origin in the 80's)





7 t_c calculation options

•	Length of Travel (Feet L)	1000
•	Slope (fraction S)	.01
•	Rainfall Intensity (inches/hour i)	4
•	Watershed Area (Square Miles A)	.1
•	Roughness Coefficient (n)	.05
•	Retardance Coefficient (Cr)	.4
•	Mannings Overland Flow Roughness (N)	.015
•	NRCS Curve Number (CN)	85
•	FAA Rational Coefficient (C)	.7

- RESULTS (time of concentration values in minutes)
- FAA 22.7 Izzard 16.5 Kerby 15.1 Kinematic 10.8 Kirpich 9.378
- Bransby Williams 12.8 NRCS 26.9

BMPTRAINS Download From:

www.stormwater.ucf.edu





WHAT'S NEW

BMPTRAINS Stormwater Best Management Practices Analysis Model (Version 5) Please register before you download the model for the first time after April 25, 2013. Registration, Model, and User's Manual.

Or After December 15, 2013 from SAMADAonline.com



NOTE>>> first time users, please enable the macros (under options)

BMP Nutrient Model BMPTRAINS

Stormwater BMP Treatment Trains [BMPTRAINS©]

CLICK HERE TO START



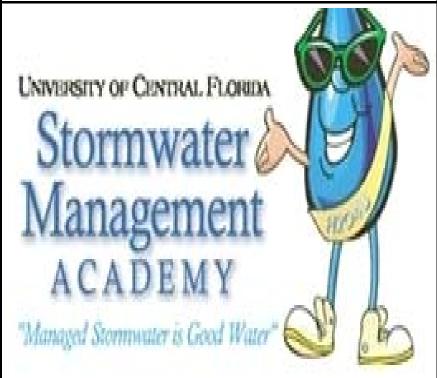


INTRODUCTION PAGE

This program is compiled from stormwater management publications and deliberations during a two year review of the stormwater rule in the State of Florida.

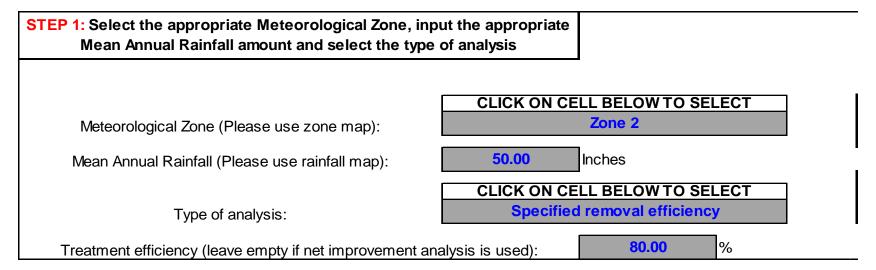
Input from the members of the Florida Department of
Environmental Protection Stormwater Review Technical Advisory Committee and the staff and consultants from the State Water Management Districts is appreciated.

The State Department of Transportation provided guidance and resources to compile this program. The Stormwater Management Academy is responsible for the content of this program.



Download from <u>www.stormwater.ucf.edu</u> and use from <u>www.smadaonline.com</u>.

BMP Rainfall Inputs



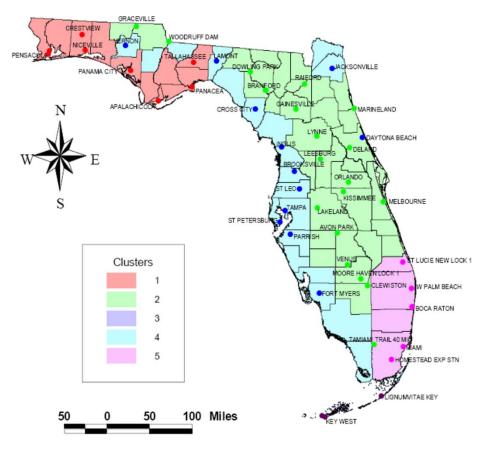
Buttons For

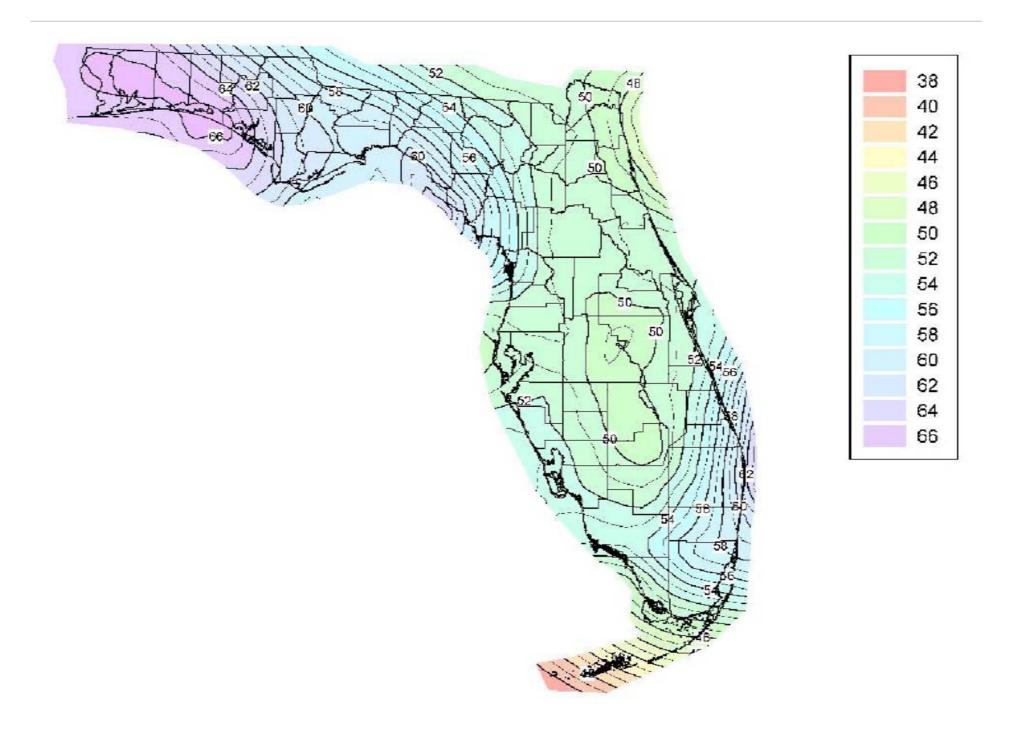
View Zone Maps

View Mean Annual Rainfall Map

Rainfall Distributions

- Average Annual Removal is the target.
- Considers rainfall distributions are regionally different.
- Selection of site date are facilitated.

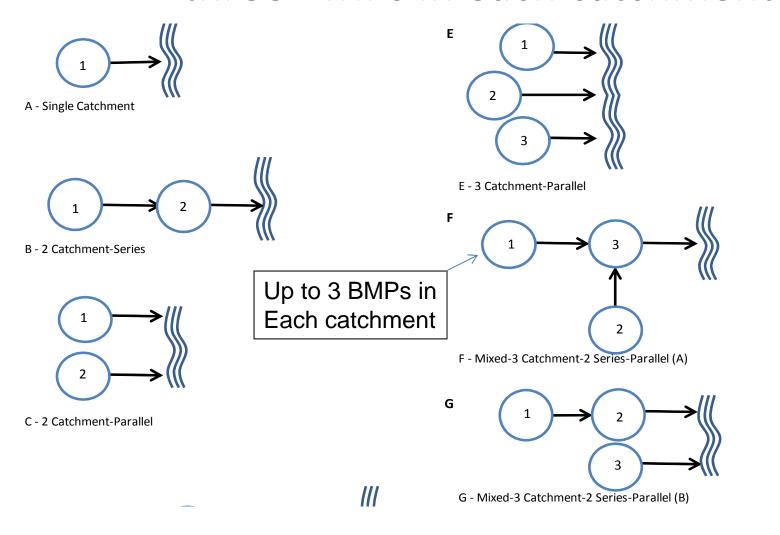




BMP Rainfall and Watershed Inputs

STEP 1: Select the appropriate Meteorological Zone, input the appropriate Mean Annual Rainfall amount and select the type of analysis **CLICK ON CELL BELOW TO SELECT** Zone 2 Meteorological Zone (Please use zone map): 50.00 Inches Mean Annual Rainfall (Please use rainfall map): **CLICK ON CELL BELOW TO SELECT** Specified removal efficiency Type of analysis: 80.00 % Treatment efficiency (leave empty if net improvement analysis is used): WATERSHED CHARACTERISTICS SINGLE SYSTEM TREATEMENT ANALYSIS WATERSHED NO.1 CHARACTERISTICS: **CLICK ON CELL BELOW TO SELECT** Pre Developed Land Use: **Undeveloped - Wet Flatwoods CLICK ON CELL BELOW TO SELECT** Post Developed Land Use: **Highway** Total pre-developed watershed area: Post-developed area watershed area: 0.55 AC Pre-Developed Non DCIA CN: 80.00 Pre-development Annua Pre-Developed DCIA Percentage: 0.00 % Pre-development Annua Post-Developed Non DCIA CN: Post-development Annu 80.00 Post-Developed DCIA Percentage: Post-development Annu

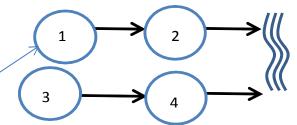
Up to four catchments and up to three BMPs in each catchment



Up to 4 catchments (up to 14 configurations)

M

Up to 3 BMPs in Each catchment with no increase in catchment area between the BMPs



M - Mixed-4 Catchment-2 Series

N - Mixed-4 Catchment-2 Series-2 Parallel

Loading Results & Change Data

	Blue Numbers =	Input data	
	Red Numbers =	Answers	
			·
Pre-development Ani	nual Mass Loading - Nitrogen:		0.369 kg/year
Pre-development Ani	nual Mass Loading - Phosphorus :		0.005kg/year
Post-development Ar	3.750 kg/year		
Post-development Ar	nnual Mass Loading - Phosphorus	0.503 kg/year	

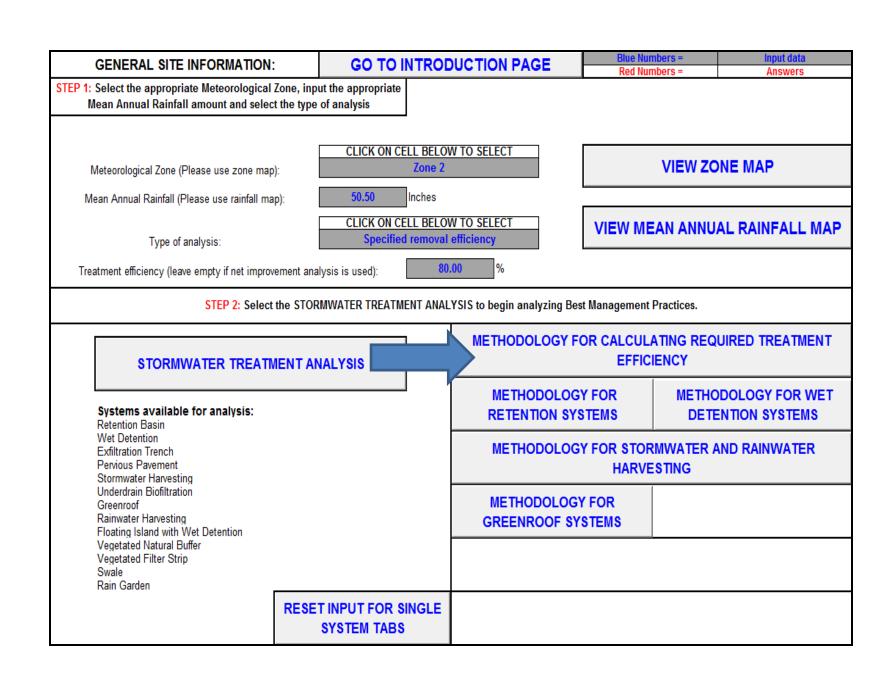
	OVERWRITE DEFAULT CONCENTRATIONS:									
	PRE:		POST:							
EMC(N):		mg/L	mg/L							
EMC(P):		mg/L	mg/L							
		•								

NOTE: If any changes to the default values are made the numbers "carry" to the end

EMC Default Values

as of June 3, 2013

	Event Mean Co	oncentration (mg/l)		
LAND USE CATEGORY	TOTAL Nitrogen	TOTAL Phosphorus		
Low-Density Residential ¹	1.51	0.178		
Single-Family	1.87	0.301		
Multi-Family	2.1	0.497		
Low-Intensity Commercial	1.07	0.179		
High-Intensity Commercial	2.2	0.248		
Light Industrial	1.19	0.213		
Highway	1.37	0.167		
Agricultural - Pasture	3.3	0.621		
Agricultural - Citrus	2.07	0.152		
Agricultural - Row Crops	2.46	0.489		
Agricultural - General Agriculture ²	2.79	0.431		
Undeveloped	1.15	0.055		
Mining / Extractive	1.18	0.15		
Average of single-family and undeveloped loading rates				
Mean of pasture, citrus, and row crop land uses				



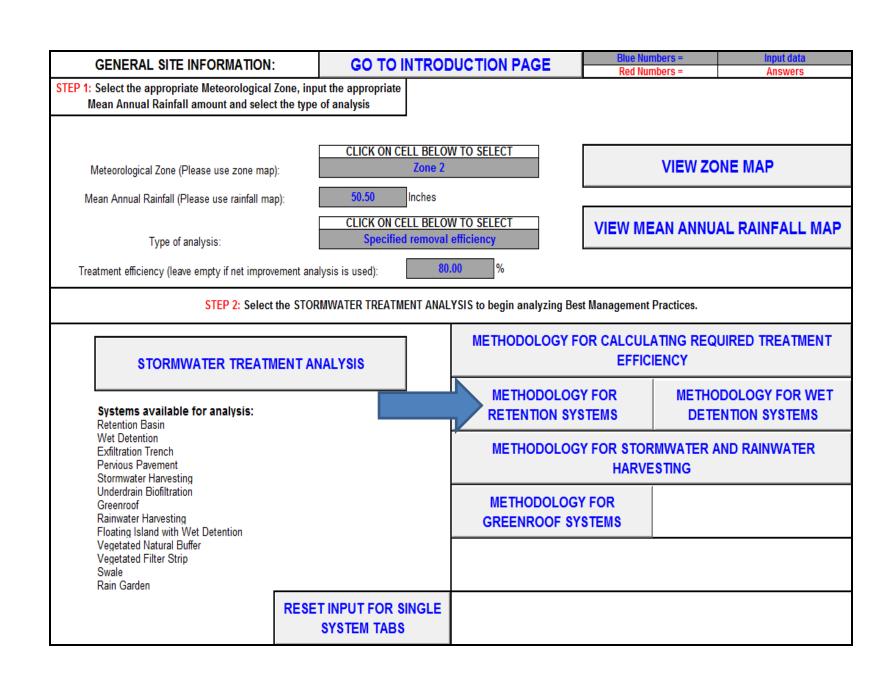
Mean Annual Runoff Coefficient

	T								Runoff		ents (C \	,									
NDCIA CN										Pe	rcent DO	CIA									
	0	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80	85	90	95	100
30	0.006	0.048	0.090	0.132	0.175	0.217	0.259	0.301	0.343	0.386	0.428	0.470	0.512	0.554	0.596	0.639	0.681	0.723	0.765	0.807	0.849
35	0.009	0.051	0.093	0.135	0.177	0.219	0.261	0.303	0.345	0.387	0.429	0.471	0.513	0.555	0.597	0.639	0.681	0.723	0.765	0.807	0.849
40	0.014	0.056	0.098	0.139	0.181	0.223	0.265	0.307	0.348	0.390	0.432	0.474	0.515	0.557	0.599	0.641	0.682	0.724	0.766	0.808	0.849
45	0.020	0.062	0.103	0.145	0.186	0.228	0.269	0.311	0.352	0.394	0.435	0.476	0.518	0.559	0.601	0.642	0.684	0.725	0.767	0.808	0.849
50	0.029	0.070	0.111	0.152	0.193	0.234	0.275	0.316	0.357	0.398	0.439	0.480	0.521	0.562	0.603	0.644	0.685	0.726	0.767	0.808	0.849
55	0.039	0.079	0.120	0.161	0.201	0.242	0.282	0.323	0.363	0.404	0.444	0.485	0.525	0.566	0.606	0.647	0.687	0.728	0.768	0.809	0.849
60	0.052	0.092	0.132	0.172	0.212	0.252	0.291	0.331	0.371	0.411	0.451	0.491	0.531	0.570	0.610	0.650	0.690	0.730	0.770	0.810	0.849
65	0.069	0.108	0.147	0.186	0.225	0.264	0.303	0.342	0.381	0.420	0.459	0.498	0.537	0.576	0.615	0.654	0.693	0.732	0.771	0.810	0.849
70	0.092	0.130	0.167	0.205	0.243	0.281	0.319	0.357	0.395	0.433	0.471	0.508	0.546	0.584	0.622	0.660	0.698	0.736	0.774	0.812	0.849
75	0.121	0.158	0.194	0.230	0.267	0.303	0.340	0.376	0.412	0.449	0.485	0.522	0.558	0.595	0.631	0.667	0.704	0.740	0.777	0.813	0.849
80	0.162	0.196	0.230	0.265	0.299	0.334	0.368	0.402	0.437	0.471	0.506	0.540	0.574	0.609	0.643	0.678	0.712	0.746	0.781	0.815	0.849
85	0.220	0.252	0.283	0.315	0.346	0.378	0.409	0.441	0.472	0.503	0.535	0.566	0.598	0.629	0.661	0.692	0.724	0.755	0.787	0.818	0.849
90	0.312	0.339	0.366	0.393	0.419	0.446	0.473	0.500	0.527	0.554	0.581	0.608	0.634	0.661	0.688	0.715	0.742	0.769	0.796	0.823	0.849
95	0.478	0.496	0.515	0.533	0.552	0.571	0.589	0.608	0.626	0.645	0.664	0.682	0.701	0.719	0.738	0.757	0.775	0.794	0.812	0.831	0.849
98	0.656	0.666	0.676	0.685	0.695	0.705	0.714	0.724	0.734	0.743	0.753	0.763	0.772	0.782	0.792	0.801	0.811	0.821	0.830	0.840	0.849

From Harper and Baker

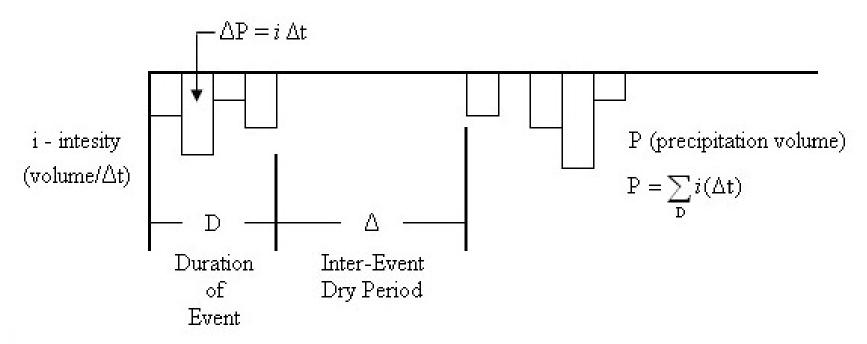
Interpolating Challenge

NDCIA CN										Pe	ercent D(CIA
	0	5	10	15	20	25	30	35	40	45	50	55
30	0.006	0.048	0.090	0.132	0.175	0.217	0.259	0.301	0.343	0.386	0.428	0.470
35	0.009	0.051	0.093	0.135	0.177	0.219	0.261	0.303	0.345	0.387	0.429	0.471
40	0.014	0.056	0.098	0.139	0.181	0.223	0.265	0.307	0.348	0.390	0.432	0.474
45	0.020	0.062	0.103	0.145	0.186	0.228	0.269	0.311	0.352	0.394	0.435	0.476
50	0.029	0.070	0.111	0.152	0.193	0.234	0.275	0.316	0.357	0.398	0.439	0.480
55	0.039	0.079	0.120	0.161	0.201	0.242	0.282	0.323	0.363	0.404	0.444	0.485



Basic Principles

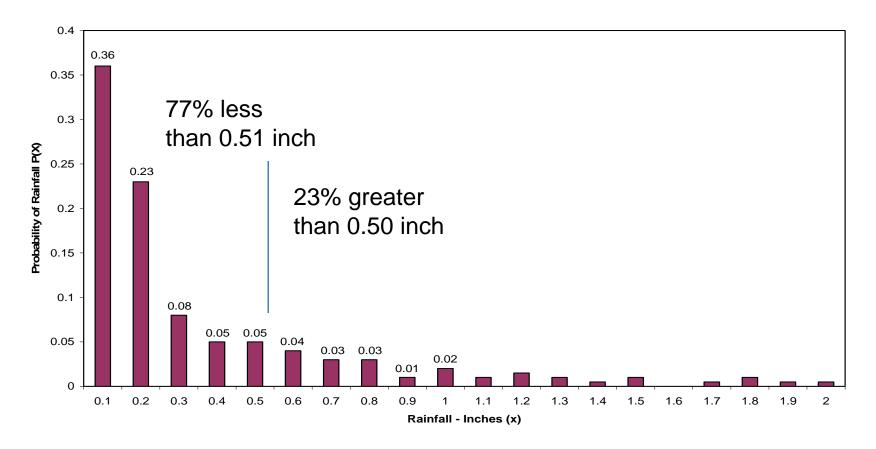
Inter-Event Dry Period



Histogram (Probability Distribution)

N=130 events per year

Histogram of Rainfall Volume - Interevent Dry Period of 4 Hours 1974 - 1989



Volume Abstracted or Diverted

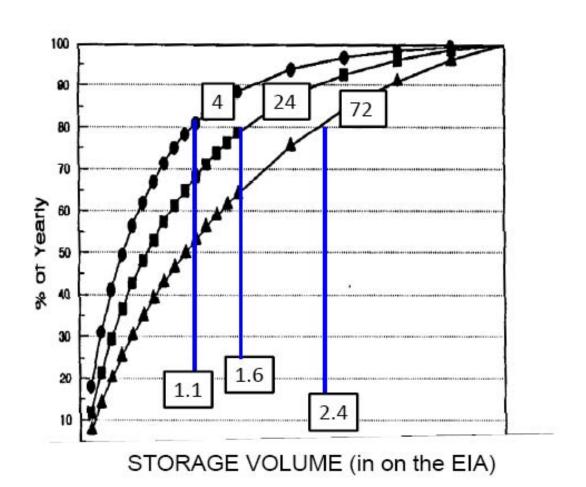
Using probability basic principles

Volume Abstracted =
$$\sum_{i}^{\text{Abstraction Vol.}} P(i)_{i} \overline{x}_{i} n + \sum_{i = \text{Abstraction Vol.}}^{\infty} P(i)_{i} (\text{Abstraction Vol.})(n)$$

Where the first term is the Expected Value of the abstraction volume up to the abstraction depth, and the second term the abstraction volume for all storm events greater than or equal to the abstraction depth.

Mass Curve

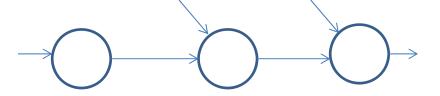
for 4, 24, and 72 hour treatment times (inter-event)



Important to note transport time and treatment time if one catchment drains through another one

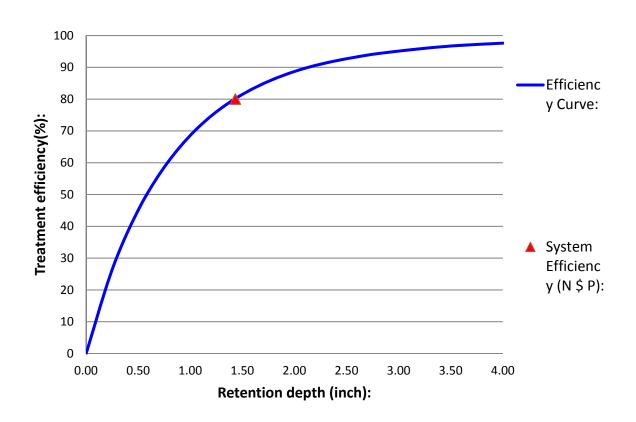
- Time of concentration
- Time for treatment (infiltration or recovery of treatment volume)
 Additional watershed area input

3 catchments



 One catchment with three BMPs (ex. pervious pave to baffle box to swale).

Example Output Retention Design



Effectiveness over the equivalent impervious area increases with the depth of retention over the area and rate of increase decreases with depth

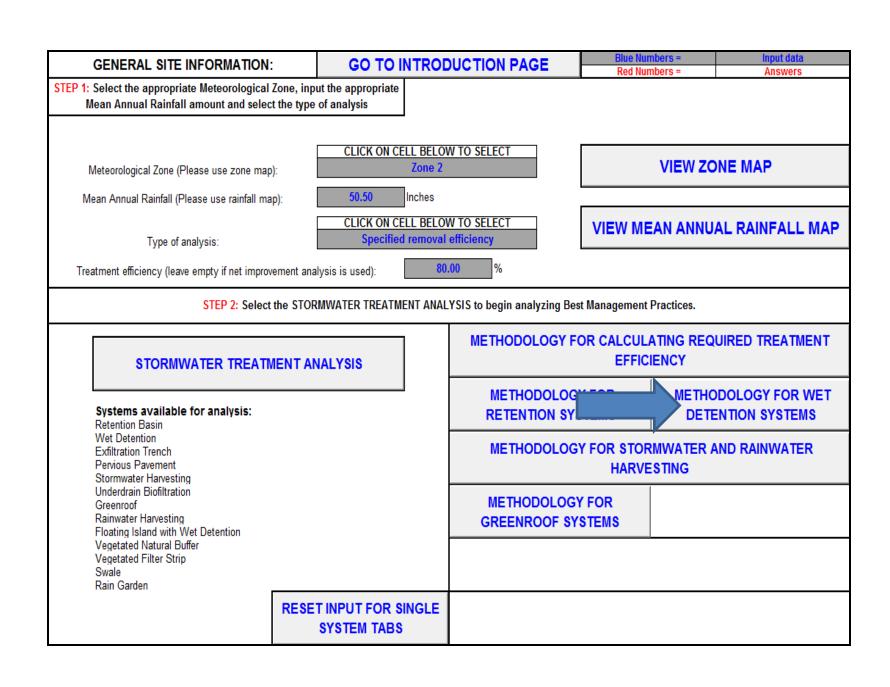
Methodology For Retention Systems

Mean Annual Mass Removal Efficiency table from Appendix D of the evaluation report (1 out of 80):

Mean Annual Mass Removal Efficiencies for 0.25-inches of Retention for Zone 1

NDCIA										Percer	nt DCIA									
CN	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80	85	90	95	100
30	86.2	81.3	73.3	65.5	58.7	53.0	48.3	44.2	40.8	37.9	35.3	33.1	31.1	29.4	27.8	26.4	25.1	24.0	22.9	21.9
35	81.6	78.7	71.7	64.5	58.0	52.5	47.9	44.0	40.6	37.7	35.2	33.0	31.0	29.3	27.8	26.4	25.1	23.9	22.9	21.9
40	76.4	75.5	69.6	63.1	57.1	51.9	47.4	43.6	40.3	37.5	35.0	32.9	30.9	29.2	27.7	26.3	25.1	23.9	22.9	21.9
45	70.7	71.7	67.2	61.4	55.9	51.0	46.8	43.1	40.0	37.2	34.8	32.7	30.8	29.1	27.6	26.3	25.0	23.9	22.9	21.9
50	64.7	67.5	64.2	59.4	54.5	50.0	46.0	42.6	39.5	36.9	34.6	32.5	30.7	29.0	27.5	26.2	25.0	23.9	22.9	21.9
55	58.6	62.8	60.9	57.0	52.7	48.7	45.1	41.8	39.0	36.5	34.2	32.3	30.5	28.9	27.4	26.1	24.9	23.9	22.9	21.9
60	52.8	57.8	57.1	54.2	50.7	47.1	43.9	40.9	38.3	35.9	33.8	31.9	30.2	28.7	27.3	26.0	24.9	23.8	22.8	21.9
65	47.3	52.6	53.0	51.1	48.3	45.3	42.5	39.8	37.4	35.3	33.3	31.5	29.9	28.4	27.1	25.9	24.8	23.8	22.8	21.9
70	42.2	47.3	48.6	47.6	45.6	43.2	40.8	38.5	36.4	34.4	32.6	31.0	29.5	28.1	26.9	25.7	24.7	23.7	22.8	21.9
75	37.8	422	43.9	43.7	42.4	40.7	38.8	36.9	35.1	33.4	31.8	30.4	29.0	27.8	26.6	25.5	24.5	23.6	22.7	21.9
80	34.0	37.5	39.1	39.4	38.8	37.7	36.4	34.9	33.5	32.1	30.8	29.5	28.3	27.2	26.2	25.2	24.3	23.5	22.7	21.9
85	30.8	33.1	34.3	34.8	34.7	34.2	33.4	32.5	31.4	30.4	29.4	28.4	27.4	26.5	25.7	24.8	24.1	23.3	22.6	21.9
90	27.9	29.2	29.9	30.3	30.3	30.2	29.8	29.3	28.8	28.2	27.5	26.8	26.2	25.5	24.9	24.2	23.6	23.0	22.5	21.9
95	25.3	25.6	25.8	25.9	26.0	25.9	25.8	25.6	25.4	25.2	24.9	24.6	24.3	24.0	23.6	23.3	23.0	22.6	22.3	21.9
98	23.8	23.8	23.8	23.7	23.7	23.6	23.5	23.4	23.3	23.2	23.1	23.0	22.9	22.8	22.6	22.5	22.4	22.2	22.1	21.9

From Harper and Baker



Methodology for Wet Detention Systems

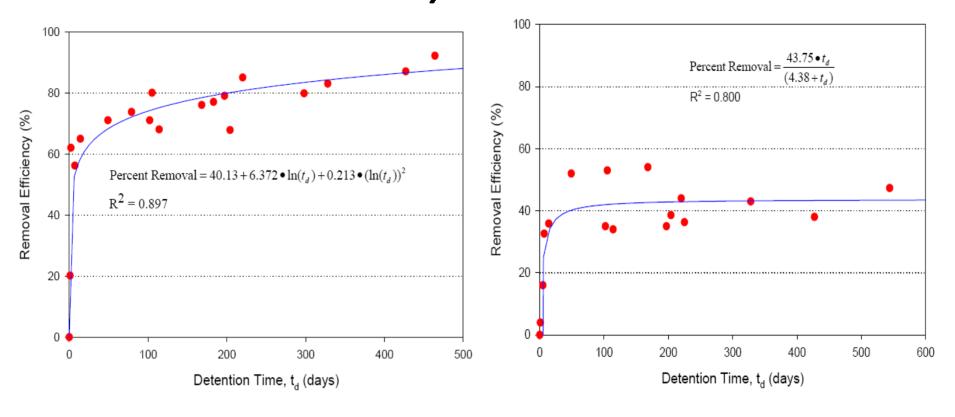


Figure 7.5-1 Removal Efficiency of Total Phosphorus in Wet Detention Ponds as a Function of Residence Time.

Figure 7.5-2 Removal Efficiency of Total Nitrogen in Wet Detention Ponds as a Function of Residence Time.

Watershed Input Data and Summaries of Mass Loadings

Pre-development non DCIA CN:	80.00		
Pre-development DCIA percentage:	0.000 %		
Annual runoff volume:	0.254 ac-ft/year		
Annual Mass Loading - Nitrogen	0.369 kg/year		
Annual Mass Loading - Phosphorus	0.005 kg/year		
•	80.00		
Post-development non DCIA CN:		Required Treatment Efficiency:	
Post-development non DCIA CN: Post-development DCIA percentage:	80.00	Required Treatment Efficiency:	
Post-development watershed characteristics: Post-development non DCIA CN: Post-development DCIA percentage: Annual runoff volume: Annual Mass Loading - Nitrogen	80.00 100.000 %	Required Treatment Efficiency: Required Treatment Eff (Nitrogen):	80.000 %

15 BMPs and One User Defined

	STEP :	2: Select one of the syst	ems below to analyze ef	ficiency.		
RETENTION BASIN	WET DETENTION	EXFILTRATION TRENCH	RAIN (BIO) GARDEN	SWALE	USER DEFINED BMP	
PERVIOUS PAVEMENT	STORMWATER HARVESTING	FILTRATION including BIOFILTRATION	LINED REUSE POND & UNDERDRAIN INPUT NOTE !!!: All individual system must be sized pr being analyzed in conjunction with other system Please read instructions in the MULTIPLE			
GREENROOF	RAINWATER HARVESTING	FLOATING ISLANDS WITH WET DETENTION			TREATMENT SYSTEMS or more information.	
VEGETATED NATURAL BUFFER	VEGETATED FILTER STRIP	VEGETATED AREA Example tree well	CATCHMENT	T AND TREATME RESULTS	ENT SUMMARY	

Retention Basins and Wet Detention Ponds are Traditional

RETENTION BASIN	WET DETENTION	EXFILTRATION TRENCH		
PERVIOUS PAVEMENT	STORMWATER HARVESTING	FILTRATION including BIOFILTRATION		
GREENROOF	RAINWATER HARVESTING	FLOATING ISLANDS WITH WET DETENTION		
VEGETATED NATURAL BUFFER	VEGETATED FILTER STRIP	TREE WELL		
RAIN (BIO) GARDEN	SWALE	USER DEFINED BMP		

RETENTION BASIN

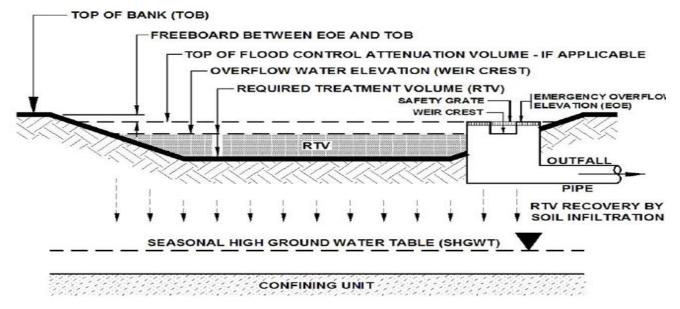
WET DETENTION

Treatment Options

RETENTION BASIN	WET DETENTION	EXFILTRATION TRENCH
PERVIOUS PAVEMENT	STORMWATER HARVESTING	UNDERDRAIN BIOFILTRATION
GREENROOF	RAINWATER HARVESTING	FLOATING ISLANDS WITH WET DETENTION
VEGETATED NATURAL BUFFER	VEGETATED FILTER STRIP	TREE WELL

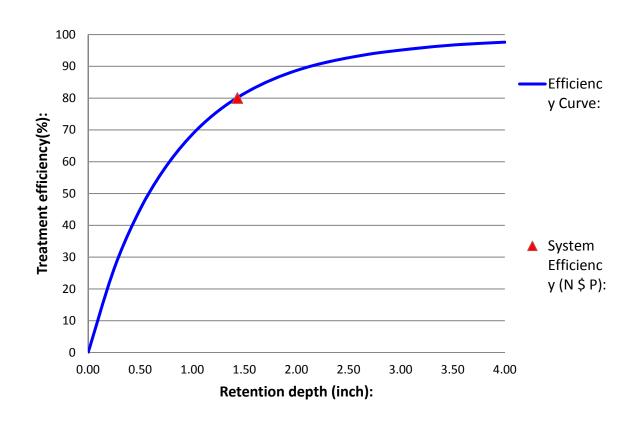
Retention Basin Size Section

DRY RETENTION BASIN SERVING ENTIRE CONTRIBUTING WATERSHED: Contributing watershed area: Required treatment efficiency: Required retention for the entire watershed to meet required efficiency: Required water quality retention volume: Required water quality retention volume: 0.55 80.00 % 1.43 in 2,864 cf Required water quality retention volume: 0.066



Example of a basin size at the bottom was 31.5' x 31.5' x 3' deep and a limiting infiltration rate of 1.5 in/hr for a 24 hour recovery time.

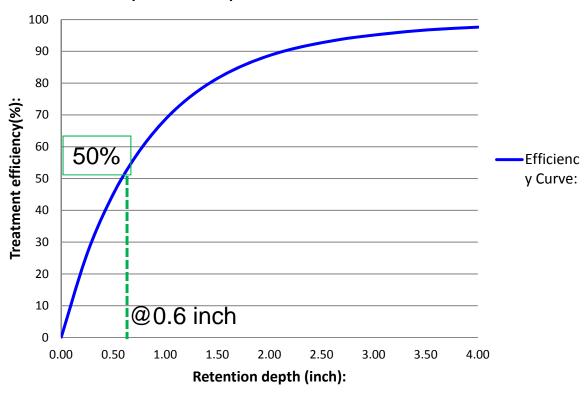
Example Output Retention Basin Design



Retention depth over the equivalent impervious area is 1.43 inches for the watershed conditions and rainfall zone.

BUT not enough area for the retention basin Thus use 3 BMPs in Series in one Watershed

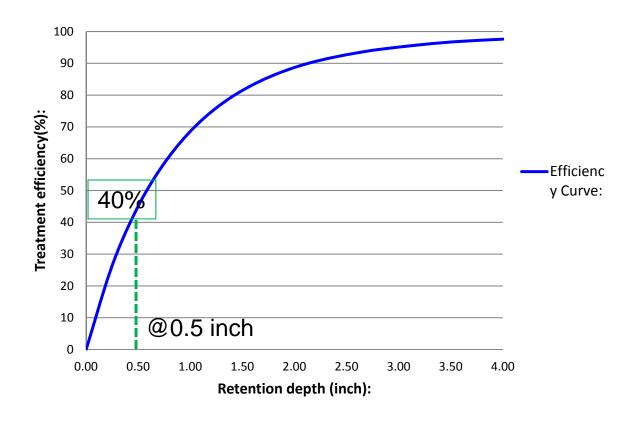
1st BMP is pervious pavement @ 0.6 inch treatment



Retention depth over the equivalent impervious area is 0.60 inches for a pervious pavement with reservoir.

Example 3 BMPs in Series in one Watershed

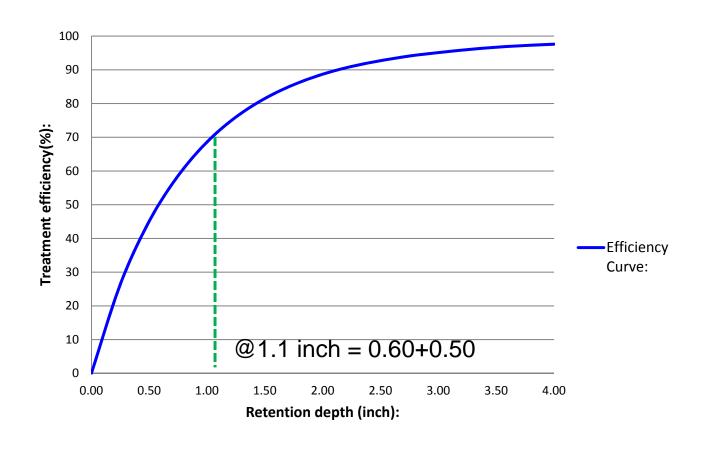
2nd BMP in series is exfiltration @ 0.5 inch treatment



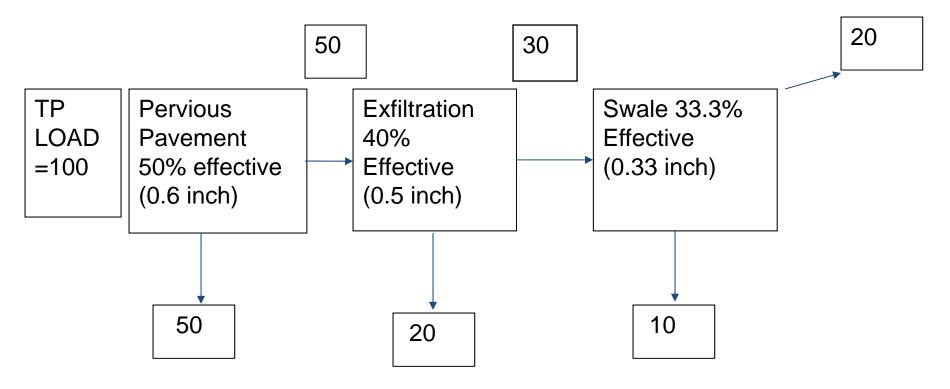
Retention depth over the equivalent impervious area is 0.50 inches for an exfiltration system.

For retention basins stay true to the underlying principles

Annual effectiveness is **not** the sum of the two efficiencies (50+40= 90%) It is however the annual effectiveness at 1.1 inch retention or 70%.



BMP TREATMENT TRAIN CREDITS more ridiculous when three efficiencies are added

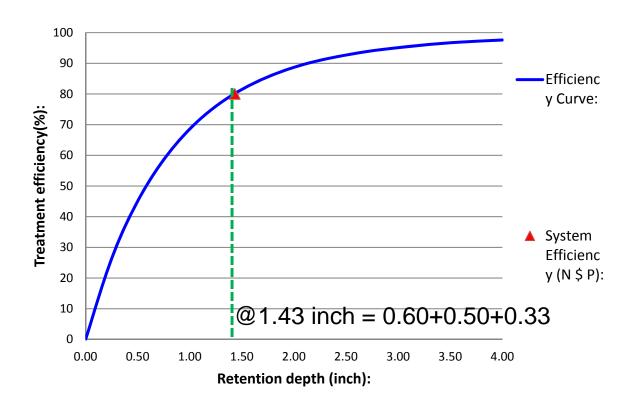


$$M = 100 [1 - {(1-0.5)(1-0.4)(1-.33)}] = 100[1-.20] = 80 \% removed$$

- NOTES 1. Example flow diagram for this problem only.
 - 2. There was no input or additional catchment flow between BMPs

3 BMPs Output Retention Design

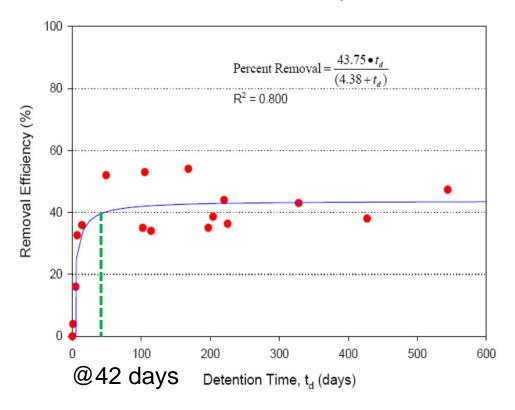
Assumes water entering ground has all pollution removed. However, the BMPTRAINS model allows an estimate of groundwater concentrations using special media blends (ex. Biosorption Activated Media).



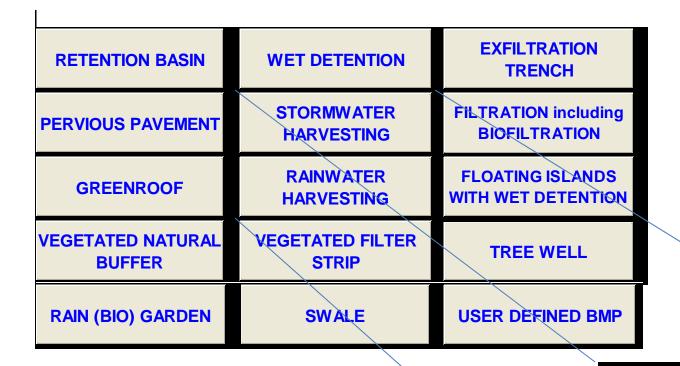
Retention depth over the equivalent impervious area is 1.43 inches.

For Wet Detention Ponds Stay true to the underlying principles

Annual effectiveness is **not** the sum of the efficiencies for 2 wet ponds each with 42 days annual residence time. They do not get 80%. The annual effectiveness at 84 days is about 42%.



Many BMP Options



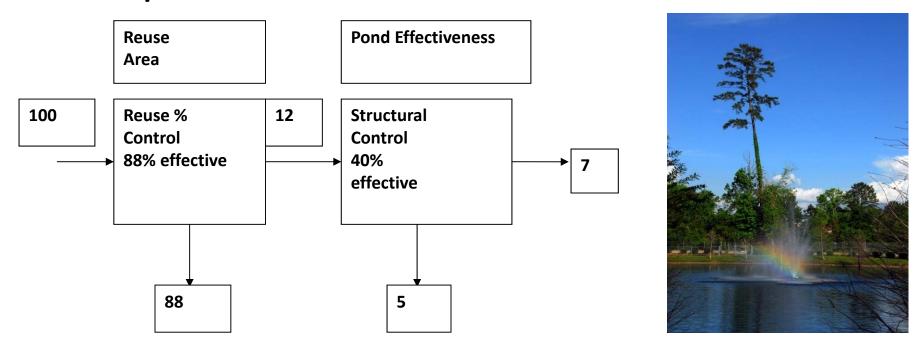
From BMPTRAINS Model (FDOT and UCF)

STORMWATER HARVESTING

RAINWATER HARVESTING

Enhanced Nitrogen Removal of a Detention Pond by Harvesting the Detained Water (note same catchment)

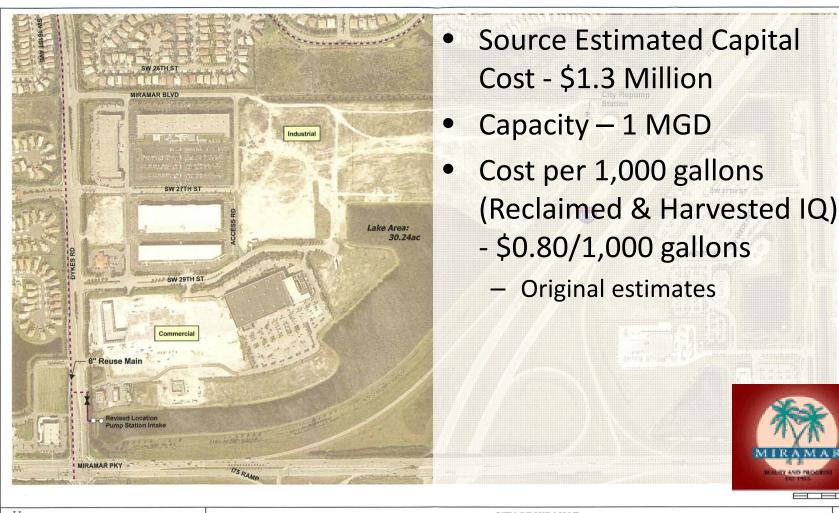
- 88% of water not discharged: N mass removed is 100[1-{(1-.88)(1-.4)}] = 93%
- 88 acre-feet supplied to 48 acres (from 24 acres, 50 inch rain, @ 88%)
- @\$2.00/1000 gallons, net revenue is about \$1000/acre irrigated/year.*
- make up water is = Need Harvested = 145.6 88 = 57.6 Acre-Feet.



^{*}Assumes a production and delivery cost of about \$0.45/1000 gallons

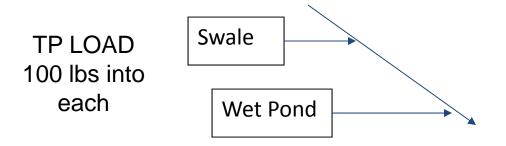
Photo Credit FDOT

4. City of Miramar – Supplement Reclaimed Water



BMP TREATMENT TRAIN CREDITS FOR PARALLEL BMPs – 2 Watersheds

Two BMPs in parallel – the discharge from each BMP goes into one system



Efficiency = (Rem₁ + Rem₂)/input sum

SWALE
35% = 35lbs
65 lbs
remaining

Wet Pond 70% = 70 lbs 30 lbs remaining

TP LOAD 95 lbs discharged From 200 input 52.5% effective



Conclusions

- 1. BMP treatment volumes are related to average annual effectiveness and are site specific.
- 2. BMP computation methods used for at least 30 years are on the "cloud". The BMPTRAIN model is available for download.
- 3. Regional rainfall and site specific watershed characteristics are used to increase the accuracy in predicting BMP performance.
- 4. BMPs can be analyzed in either series or parallel structure. Stay "true" to the underlying principles.
- 5. Computation aids are available and are being used in Florida.







Best Management Practices and Making BMPTRAINS

Marty Wanielista

Questions and Discussion Thank You...

www.stormwater.ucf.edu





